

SAIHAM KNIT COMPOSITE LTD. (Extension 2)

Saiham Nagar, Noyapara, Madhabpur, Hobigonj

(24.182973, 91.354747)

16 January 2025

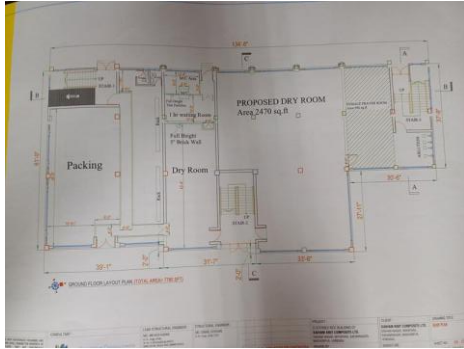


1. Building Information

- 1) Locker Building: Two-storied (G+1) reinforced concrete (RC) structure.
- 2) Printing Building: Two-storied (G+1) reinforced concrete (RC) structure.
- 3) AOP Building: Single-storied reinforced concrete (RC) structure with a mezzanine floor.
- 4) Knitting Building: Three-storied (G+2) reinforced concrete (RC) structure.
- 5) Dyeing Building: Four-storied (G+M+3) reinforced concrete (RC) structure with a mezzanine.
- 6) Yarn Warehouse Shed: Single-storied steel shed.
- 7) RMS Room: Single-storied RC structure.
- 8) New Generator: Single-storied RC structure
- 9) Hydrogen Peroxide Building: Single-storied RC structure
- 10) ETP-3: Single-storied reinforced concrete structure with a roof top lab room.
- 11) Connecting Bridge: Single-storied steel bridge.

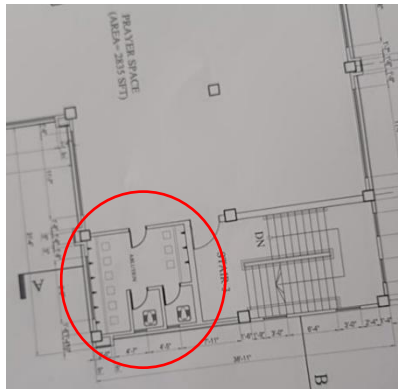
2. Observations

Observation-1: Design report needs to be reviewed against lateral forces (Locker Building).



Description: The prepared design report needs to be reviewed against lateral forces. Submit the design report to the RSC for detailed review.

Observation-2: Mismatch between drawing & onsite condition (Locker Building).



Description: The floor rise, and toilet zone brick wall layout didn't match with drawing. Also, the rooftop PVC tank wasn't shown in the drawing. The building engineer is required to update the drawing based on the as-constructed structure.

Observation-3: Lack of anchorage to the storage racks (Locker Building).



Description: Lack of anchorage/brace was observed for the storage racks. The building engineer is required to anchor/brace all the non-structural elements within the building.

Observation-4: Inconsistency and lack of information in the design report.
(Printing Building, Knitting Building).

The screenshot shows the ASCE 7-10 Seismic Loading software interface. The 'Direction and Eccentricity' section has 'X Dir' selected. The 'Seismic Coefficients' section shows 'Sa and S1 from USGS Database - by Latitude/Longitude' selected. The 'Time Period' section shows 'User Defined' with 'T = 0.367 sec'. The 'Story Range' section shows 'Top Story for Seismic Loads' as 'ROOF TOP' and 'Bottom Story for Seismic Loads' as 'Base'. The 'Factors' section shows 'Response Modification, R' as 8 (highlighted with a red box), 'System Overstrength, Omega' as 3, 'Deflection Amplification, Cd' as 5.5, and 'Occupancy Importance, I' as 1. The 'Drift Check for Earthquake Load in the Direction of X' table shows results for 3F ROOF, 1F, and GF stories. The 'Drift Check for Earthquake Load in the Direction of Y' table shows results for 3F ROOF, 1F, and GF stories. The '5.2 Torsional Irregularity Check' section includes a table for 'Table 5.1 Torsional Irregularity Check in X-Direction'.

Story	Height(h)	R	Allowable Inter-story Drift(inch)	As Per NBC (Earthquake Load) Deflection(inch)	Story Drift(inch)	Verdict
3F ROOF	12	8	3.600	1.012	5.56	OK
1F	12	8	3.600	0.740	4.07	OK
GF	8	8	3.600	0.362	1.44	OK

Story	Height(h)	R	Allowable Inter-story Drift(inch)	As Per NBC (Earthquake Load) Deflection(inch)	Story Drift(inch)	Verdict
3F ROOF	12	8	3.600	1.280	7.04	OK
1F	12	8	3.600	0.923	5.08	OK
GF	8	8	3.600	0.329	1.81	OK

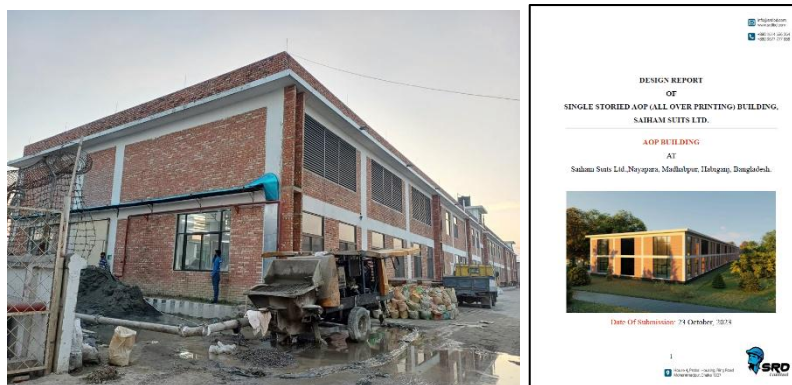
Story	Load Case/Combo	Direction	Maximum in	Average in	Ratio
ROOF TOP	EQX	X	1.148415	1.103527	1.041
ROOF	EQX	X	1.010958	0.991517	1.02
1F	EQX	X	0.739975	0.727344	1.027

SMF detailing & structural irregularity checks were not incorporated in the provided design report.

Description: The building engineer has prepared the design report based on Special Reinforced Concrete Moment Frames (SMF). However, SMF detailing checks have not been addressed in the report as per BNBC 2020 Part VI, Section 8.3. The building engineer must incorporate the check in compliance with all SMF detailing requirements outlined in BNBC 2020 Sections 8.3.4, 8.3.5, 8.3.7, 8.3.8, and 8.3.11. The building engineer has not incorporated the horizontal and vertical irregularity checks. Additionally, the structure also features re-entrant corners therefore, all horizontal and vertical irregularity checks must be included in the report.

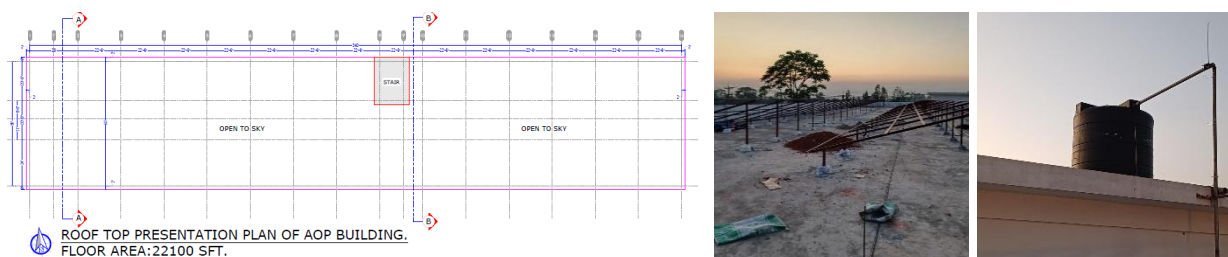
These checks should be verified and incorporated into the design report to ensure full alignment with the code provisions. Submit the revised design report to RSC for review.

Observation-5: Design report needs to be reviewed against lateral forces (AOP Building).



Description: The design report prepared needs to be reviewed against lateral forces. Submit the design report to the RSC for detailed review.

Observation-6: Mismatch between drawing & onsite condition (AOP Building).



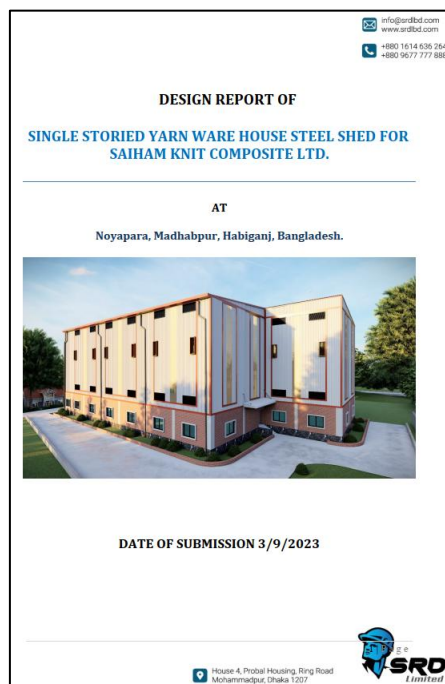
Description: The rooftop PVC tank and solar panels weren't shown in the architectural drawing. The building engineer is required to update the drawing based on the as-constructed structure.

Observation-7: Lack of anchorage to the storage racks (AOP Building).



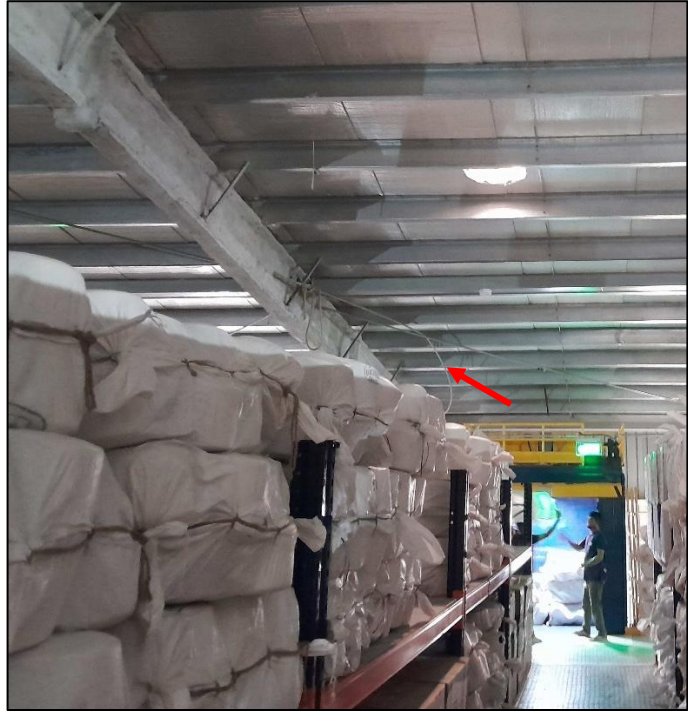
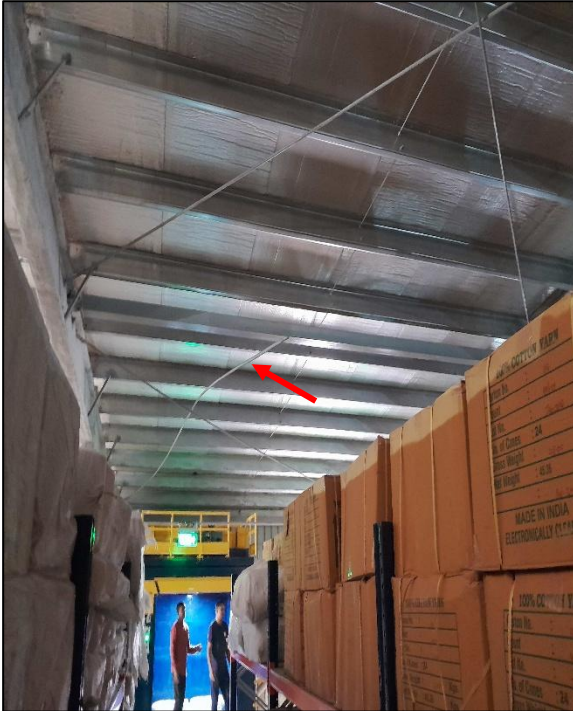
Description: Lack of anchorage/brace observed for the storage racks. The building engineer is required to anchor/brace all the non-structural elements within the building.

Observation-8: Design report needs to be reviewed against lateral forces (Yarn Warehouse).



Description: The design report prepared needs to be reviewed against lateral forces. Submit the design report to the RSC for detailed review.

Observation-9: Disconnected roof bracing (Yarn Warehouse).



Description: During the inspection, disconnected roof bracings were observed at several locations. The factory is required to connect all the roof bracings appropriately.

Observation-10: Connection gap on steel plates (Yarn Warehouse).



Description: During the inspection, connection gaps were observed in rafter-column joints. The factory is required to tighten up the connection joints or fill the connection gap with shim plate if required.

Observation-11: Long-height storage racks (Yarn Warehouse).



Description: During the inspection, approximately 15m high free-standing, self-supported storage racks were found in the building. However, no design report was available. The building engineer is required to prepare an Engineering Assessment (EA) report to confirm the stability of the storage racks.

Observation-12: Inadequate Rack anchorage (Yarn Warehouse).



Description: During the inspection, long height storage racks were found partially anchored. The factory has not anchored all proposed points. The factory is required to anchor adequately all proposed points.

Observation-13: Inconsistency and lack of information in the design report (Dyeing Building).

DESIGN REPORT OF FOUR STORED DYEING BUILDING							
Story	Load Case/Combo	Direction	Max Drift (mm)	Avg Drift (mm)	Ratio	Limit	Remarks
ROOF	EQY 1	Y	0.330501	0.309722	1.035	1.2	Within Limit
ROOF	EQY 2	Y	0.343694	0.313039	1.098	1.2	Within Limit
ROOF	EQY 3	Y	0.358618	0.306406	1.17	1.2	Within Limit
3F	EQX 1	X	0.451542	0.426175	1.06	1.2	Within Limit
3F	EQX 2	X	0.446748	0.425879	1.049	1.2	Within Limit
3F	EQX 3	X	0.498076	0.426471	1.168	1.2	Within Limit
3F	EQY 1	Y	0.457043	0.444085	1.037	1.2	Within Limit
3F	EQY 2	Y	0.489265	0.44457	1.101	1.2	Within Limit
3F	EQY 3	Y	0.514211	0.437131	1.176	1.2	Within Limit
2F	EQX 1	X	0.539525	0.508022	1.062	1.2	Within Limit
2F	EQX 2	X	0.530661	0.507164	1.046	1.2	Within Limit
2F	EQX 3	X	0.595883	0.508881	1.17	1.2	Within Limit
2F	EQY 1	Y	0.539324	0.52106	1.035	1.2	Within Limit
2F	EQY 2	Y	0.578825	0.52474	1.103	1.2	Within Limit
2F	EQY 3	Y	0.607993	0.517381	1.175	1.2	Within Limit
1F	EQX 1	X	0.430162	0.370105	1.162	1.2	Within Limit
1F	EQX 2	X	0.422185	0.368655	1.145	1.2	Within Limit
1F	EQX 3	X	0.473833	0.378928	1.25	1.2	Within Limit
1F	EQY 1	Y	0.428455	0.372729	1.14	1.2	Within Limit
1F	EQY 2	Y	0.460801	0.383478	1.202	1.2	Within Limit
1F	EQY 3	Y	0.48235	0.38495	1.253	1.2	Within Limit
TIE	EQX 1	X	0.327189	0.307116	1.065	1.2	Within Limit
TIE	EQX 2	X	0.319887	0.30923	1.048	1.2	Within Limit
TIE	EQX 3	X	0.356956	0.306507	1.165	1.2	Within Limit
TIE	EQY 1	Y	0.323746	0.309521	1.046	1.2	Within Limit
TIE	EQY 2	Y	0.344423	0.311761	1.105	1.2	Within Limit
TIE	EQY 3	Y	0.363047	0.309401	1.173	1.2	Within Limit
GF	EQX 1	X	0.014025	0.012605	1.113	1.2	Within Limit
GF	EQX 2	X	0.013566	0.012503	1.085	1.2	Within Limit
GF	EQX 3	X	0.015238	0.012767	1.197	1.2	Within Limit
GF	EQY 1	Y	0.01355	0.012607	1.075	1.2	Within Limit
GF	EQY 2	Y	0.014308	0.012769	1.12	1.2	Within Limit
GF	EQY 3	Y	0.015193	0.012659	1.2	1.2	Within Limit

Comment: It has been found that the Building has no any Torsional Irregularity.

Torsional irregularity exists in the structure.

Structural Type --> Seismic Force Resisting System --> Percentage of critical damping--> Response Reduction Factor(R)-->	MOMENT_RESISTING_FRAME_SYSTEMS_ no_shear_wall	Coefficients to Estimate Approximate Period		
	4. Special reinforced concrete moment frames	Structure type	Ct	m
	5%	CMRF	0.0466	0.9
	8			

DESIGN REPORT OF FOUR STORED DYEING BUILDING

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DESIGN REPORT OF FOUR STORED DYEING BUILDING

7.5 TORSIONAL IRREGULARITY CHECK

To be considered for rigid floor diaphragms, when the maximum storey drift (Δ_{max}) as shown in Figure 6.2.27(a), computed including accidental torsion, at one end of the structure is more than 1.2 times the average ($\Delta_{avg} = \frac{\Delta_{max} + \Delta_{min}}{2}$) of the storey drifts at the two ends of the structure. If $\Delta_{max} > 1.4\Delta_{avg}$ then the irregularity is termed as extreme torsional irregularity.

Story	Joint Label	Load Case/Combo	FZ (Kip)	No. of Pile Provided	Load Per Pile (Kip)	Allowable Bearing Capacity of Pile (Kip)	Remarks
Base	18	D+L	938.655	6	156.475833	210	-53.52417
Base	19	D+L	837.673	6	139.612167	210	-70.38783
Base	67	D+L	943.674	6	157.279	210	-52.721
Base	68	D+L	687.39	5	138.48	210	-70.402
Base	70	D+L	723.707	5	144.7414	210	-65.2586
Base	71	D+L	665.134	5	133.0268	210	-76.9732
Base	72	D+L	653.39	5	130.678	210	-79.322
Base	73	D+L	1007.6	6	167.933333	210	-42.06667
Base	74	D+L	1015.018	6	169.169667	210	-40.83033
Base	75	D+L	819.539	5	163.9078	210	-46.0922
Base	76	D+L	788.688	5	157.7376	210	-52.2624
Base	77	D+L	787.064	5	157.4128	210	-52.5872
Base	364	D+L	975.213	6	162.5355	210	-47.4645
Base	365	D+L	1283.671	8	160.458875	210	-49.54113
Base	366	D+L	979.256	6	163.209333	210	-46.79067
Base	367	D+L	925.732	6	154.288667	210	-55.71133
Base	368	D+L	929.36	6	154.893333	210	-55.10667
Base	369	D+L	616.029	4	154.00725	210	-55.99275
Base	370	D+L	760.872	5	152.1744	210	-57.8256
Base	371	D+L	762.378	5	152.4756	210	-57.5244
Base	372	D+L	760.02	5	152.004	210	-57.396
Base	373	D+L	759.831	5	151.9662	210	-58.0338
Base	374	D+L	761.436	5	152.2872	210	-57.7128

The Provided Reinforcements are greater than the Required Reinforcements

DESIGN REPORT OF FOUR STORED DYEING BUILDING

Story	Load Case/Combo	Direction	Max Drift (in)	Avg Drift (in)	Ratio	Limit	Remarks
ROOF TOP	EQX 1	X	0.353482	0.319367	1.107	1.2	Within Limit
ROOF TOP	EQX 2	X	0.338533	0.304395	1.112	1.2	Within Limit
ROOF TOP	EQX 3	X	0.379339	0.319146	1.189	1.2	Within Limit
ROOF TOP	EQY 1	Y	0.37078	0.3352	1.106	1.2	Within Limit
ROOF TOP	EQY 2	Y	0.355965	0.329357	1.081	1.2	Within Limit
ROOF TOP	EQY 3	Y	0.408909	0.340355	1.201	1.2	Within Limit
ROOF	EQX 1	X	0.308609	0.295518	1.044	1.2	Within Limit
ROOF	EQX 2	X	0.314224	0.295935	1.062	1.2	Within Limit
ROOF	EQX 3	X	0.339573	0.295102	1.151	1.2	Within Limit

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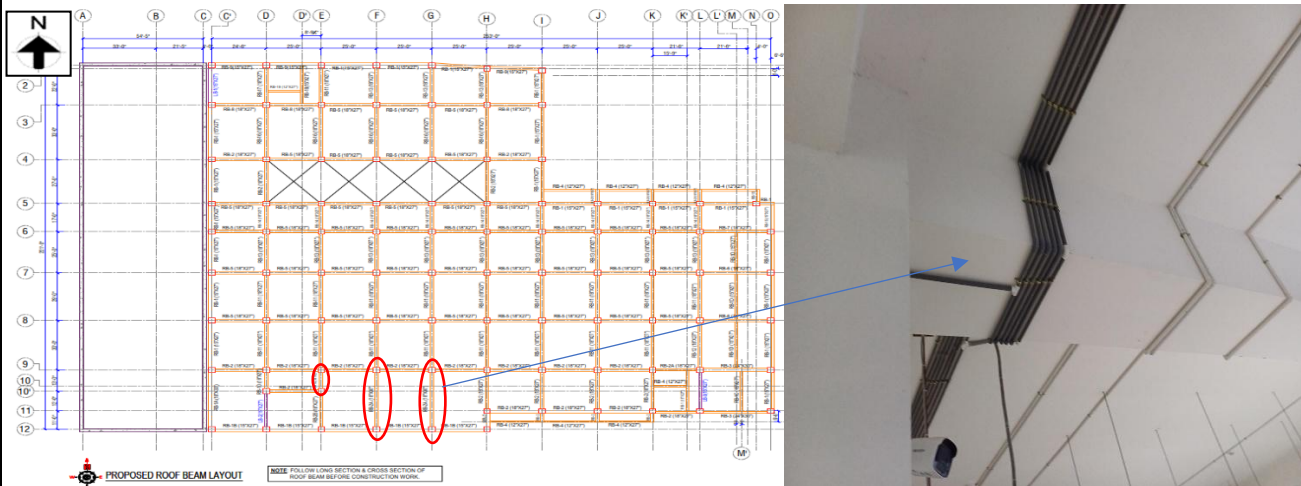
DESIGN REPORT OF FOUR STORED DYEING BUILDING

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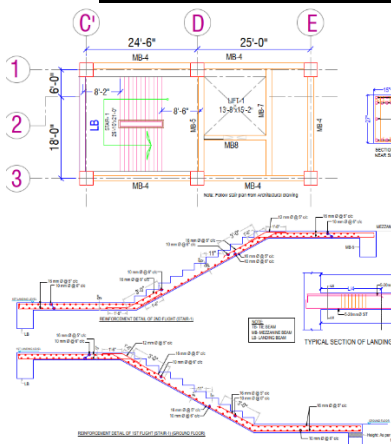
SMF detailing, beam adequacy, pile bearing capacity, structural irregularity etc. checks were not incorporated in the provided design report.

Description: The torsional irregularity exists in the structure. It shall be accounted as BNBC 2020 Part-VI Section 2.5.7.6.2. Also, the building engineer has prepared the design report based on Special Reinforced Concrete Moment Frames (SMF). However, SMF detailing checks have not been addressed in the report as per BNBC 2020 Part VI, Section 8.3. The building engineer must incorporate the check in compliance with all SMF detailing requirements outlined in BNBC 2020 Sections 8.3.4, 8.3.5, 8.3.7, 8.3.8, and 8.3.11. Additionally, the geotechnical investigation report does not recommend the $\phi 600$ pile capacity, and the pile bearing capacity check must be included in the design report. The structure also features re-entrant corners and diaphragm discontinuity; therefore, all horizontal and vertical irregularity checks must be included in the report. These checks should be verified and incorporated into the design report to ensure full alignment with the code provisions. Submit the revised design report to RSC for review.

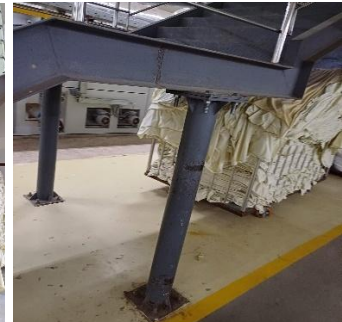
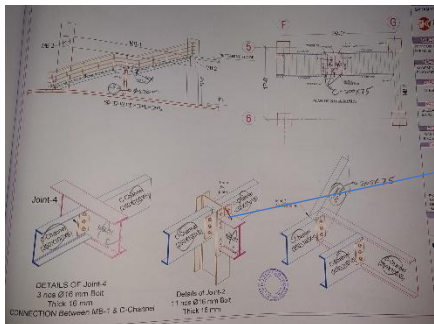
Observation-14: Discrepancy and lack of information in provided drawing (Dyeing Building).



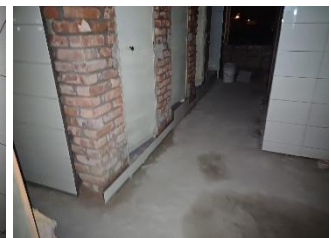
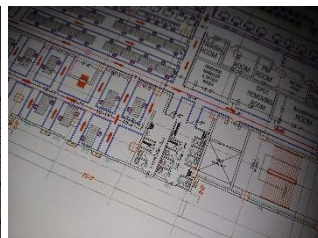
Mentioned roof beam width 600mm instead of 450mm as per the provided drawing.



14 no's stair tread found instead of 10 no's in the one flight. Also, mentioned tread riser size mismatching as mentioned in the drawing.



Pipe section and c-channel found instead of I-section. Also, connection detail mismatches.



Floor raises were found in the toilet zone which were not mentioned in the drawing.

Description: During the inspection, discrepancies and lack of information were observed between the provided drawing and on-site conditions. The building engineer is required to survey the whole structure and update the drawing reflecting actual on-site condition.

Observation-15: Frequency of material testing (Dyeing Building).

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Description: During the inspection, a total of 12 sets of concrete cylinder test reports (foundation = 3 sets; column = 3 sets; beam-slab = 6 sets) were provided. The building engineer is required to verify the frequency of testing requirements as per BNBC (Part VI, Chapter 5, 5.12.2) and update the analysis & design report if required. Also, verify in-situ material strength by further testing.

Observation-16: Crack on brick wall (Dyeing Building).



Description: During the inspection, brick wall cracks were observed on the ground floor peripheral wall. The factory is required to repair the cracks as per the recommendation of the building engineer.

Observation-17: Trolley impact on column surface (Dyeing Building).



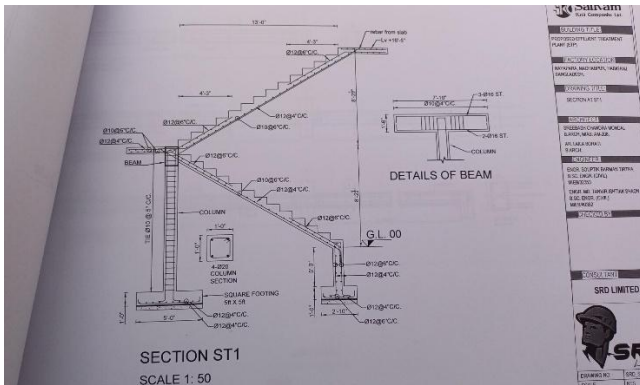
Description: The cover of floor columns was damaged due to trolley impact. The building engineer is required to provide column/wall guard to protect from trolley impact.

Observation-18: Possible falling hazard (Dyeing Building).



Description: During the inspection, it was noted that toilet zone peripheral lacked barriers at its edges, creating a potential falling hazard. The factory is required to install appropriate safety barriers to mitigate the risk of falls and ensure compliance with safety standards.

Observation-19: Mismatch and lack of information in provided drawing (ETP-3).



Steel stair found instead of RC stair as per the provided drawing



ETP roof top lab room & steel roof structural details were not shown in the provided drawing

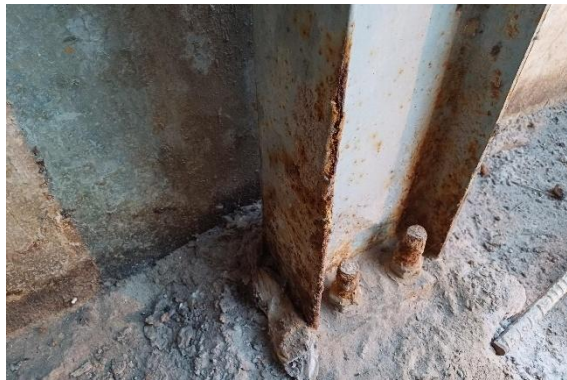
Description: During the inspection, discrepancies and lack of information were observed between the provided drawing and on-site conditions. The building engineer is required to survey the whole structure and update the drawing reflecting actual on-site condition.

Observation-20: Inadequate steel roof and stair (ETP-3).



Description: The size of steel roof members was observed inadequate to resist the vertical and lateral loading for the mentioned structure. The building engineer is required to suggest adequate size of steel members, develop adequate connection for the structure. Submit the adequacy check calculation and remedial drawing to RSC as part of safety check report.

Observation-21: Corrosion on steel member (ETP-3).



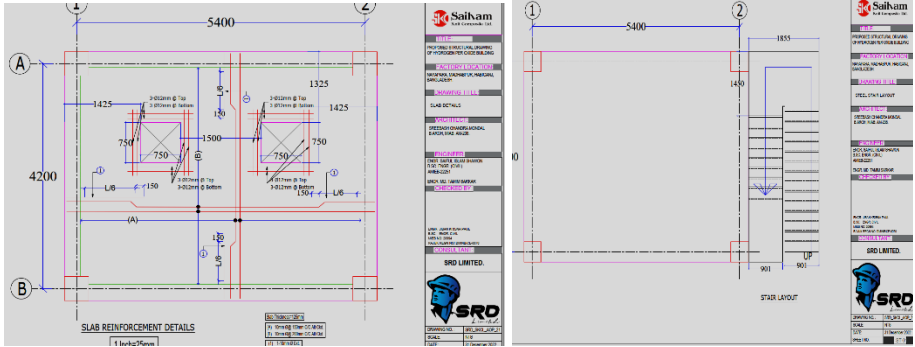
Description: During the inspection, corrosion was observed on the steel column. The factory is required to repair/replace the corroded member and apply anti-corrosive coating.

Observation-22: Vibration in lab room supporting slab (ETP-3).



Description: During the inspection, vibration was felt on the ETP lab room supporting slab. The building engineer is required to check the reason for the vibration, check the slab adequacy subject to imposed loading & vibration and suggest proper remedial/damping methodology.

Observation-23: Lack of information in provided drawing (Hydrogen Peroxide Building).



Cantilever slab on peripheries has not been shown, and the proposed stair has not been constructed.

Description: During the inspection, lack of information was observed between the provided drawing and on-site conditions. The building engineer is required to survey the whole structure and prepare as-built drawing reflecting actual on-site condition.

Observation-24: Lack of information in provided drawing (New Generator).



Roof top RC frames and details and two-story steel platform are not shown in the provided drawing

Description: During the inspection, lack of information was observed between the provided drawing and on-site conditions. The building engineer is required to survey the whole structure and prepare as-built drawing reflecting actual on-site condition.

Observation-25: Inadequate steel member sizes (New Generator).



Description: The size of steel members was observed to be inadequate to resist the vertical and lateral loading for the mentioned structure. The building engineer is required to suggest adequate size of steel members, develop the adequate connection for the structure. Submit the adequacy check calculation and remedial drawing as part of safety check report.

Observation-26: Corrosion on steel member (Connecting Bridge).



Description: During the inspection, corrosion was observed on the steel column. The factory is required to repair/replace the corroded member and apply anti-corrosive coating.

Observation-27: Distorted steel column (Connecting Bridge).



Description: During the inspection, distorted steel column flange was observed in several locations. The building engineer is required to repair the damaged steel column flange with suitable method.

3. Action Plan

Item No	Observation	Action Plan	Timeline
1.	Design report needs to be reviewed against lateral forces (Locker Building).	The building engineer is required to revise the design report and submit it to RSC for review against lateral forces.	within 6 weeks
2.		Carry out remedial work if required.	within 6 months
3.	Mismatch between drawing & onsite condition (Locker Building).	The building engineer is required to update the drawing to the as-constructed status.	within 6 weeks
4.	Lack of anchorage to the storage racks (Locker Building).	The building engineer is required to anchor/brace all the non-structural elements within the building.	within 6 months
5.	Inconsistency and lack of information in the design report (Printing Building).	The building engineer is required to revise the design report and submit it to RSC for review.	within 6 weeks
6.		Carry out remedial work if required.	within 6 months
7.	Design report needs to be reviewed against lateral forces (AOP Building).	The building engineer is required to revise the design report and submit it to RSC for review against lateral forces.	within 6 weeks
8.		Carry out remedial work if required.	within 6 months
9.	Mismatch between drawing & onsite condition (AOP Building).	The building engineer is required to update the drawing to the as-constructed status.	within 6 weeks
10.	Lack of anchorage to the storage racks (AOP Building).	The building engineer is required to anchor/brace all the non-structural elements within the building.	within 6 months
11.	Inconsistency and lack of information in the design report. (Knitting Building).	The building engineer is required to revise the design report and submit it to RSC for review.	within 6 weeks
12.		Carry out remedial work if required.	within 6 months
13.	Inconsistency and lack of information in the design report (Dyeing Building).	The building engineer is required to revise the design report and submit it to RSC for review.	within 6 weeks
14.		Carry out remedial work if required.	within 6 months
15.	Discrepancy and lack of information in provided drawing (Dyeing Building).	The building engineer is required to survey the whole structure and update the drawing reflecting actual on-site condition.	within 6 weeks

Item No	Observation	Action Plan	Timeline
16.	Frequency of material testing (Dyeing Building).	The building engineer is required to verify the frequency of testing requirements as per BNBC (Part VI, Section 5.12.2) and update the analysis & design report if required. Also, verify in-situ material strength by further testing.	within 6 weeks
17.	Crack on brick wall (Dyeing Building).	The factory is required to repair the cracks as per the recommendation of the building engineer.	within 6 months
18.	Trolley impact on column surface (Dyeing Building).	The building engineer is required to provide column/wall guard to protect the trolley impact.	within 6 months
19.	Possible falling hazard (Dyeing Building).	The factory is required to install appropriate safety barriers to mitigate the risk of falls and ensure compliance with safety standards.	within 6 weeks
20.	Design report needs to be reviewed against lateral forces (Yarn Warehouse).	The building engineer is required to revise the design report and submit it to RSC for review against lateral forces.	within 6 weeks
21.		Carry out remedial work if required.	within 6 months
22.	Disconnected roof bracing (Yarn Warehouse).	The factory is required to connect all the roof bracings appropriately.	within 6 weeks
23.	Connection gap on steel plates (Yarn Warehouse).	The factory is required to tighten up the connection joints or fill the connection gap with shim plate if required.	within 6 weeks
24.	Long height storage racks (Yarn Warehouse).	The building engineer is required to prepare an Engineering Assessment (EA) report to confirm the stability of the storage racks.	within 6 weeks
25.		Carry out remedial work if required.	within 6 months
26.	Inadequate Rack anchorage (Yarn Warehouse).	The factory is required to anchor adequately all proposed points.	within 6 weeks
27.	Mismatch and lack of information in provided drawing (ETP-3).	The building engineer is required to survey the whole structure and update the drawing reflecting actual on-site condition.	within 6 weeks
28.	Inadequate steel roof and stair (ETP-3).	The building engineer is required to suggest adequate size of steel members, develop adequate connections for the structures. Submit the adequacy check calculation and remedial drawing as part of safety check report.	within 6 weeks
29.		Carry out remedial works.	within 6 months
30.	Corrosion on steel member (ETP-3).	The factory is required to repair/replace the corroded member and apply anti-corrosive coating.	within 6 weeks

Item No	Observation	Action Plan	Timeline
31.	Vibration on lab room (ETP-3).	The building engineer is required to check the reason for the vibration, check the slab adequacy subject to imposed loading & vibration and suggest proper remedial/damping methodology.	within 6 weeks
32.		Carry out remedial works.	within 6 months
33.	Lack of information in provided drawing (Hydrogen Peroxide Building).	The building engineer is required to survey the whole structure and prepare as-built drawing reflecting actual on-site condition.	within 6 weeks
34.	Lack of information in provided drawing (New Generator).	The building engineer is required to survey the whole structure and prepare as-built drawing reflecting actual on-site condition.	within 6 weeks
35.	Inadequate steel member sizes (New Generator).	The building engineer is required to suggest adequate size of steel members, develop adequate connections for the structures. Submit the adequacy check calculation and remedial drawing as part of safety check report.	within 6 weeks
36.		Carry out remedial works.	within 6 months
37.	Corrosion on steel member (Connecting Bridge).	The factory is required to repair/replace the corroded member and apply anti-corrosive coating.	within 6 weeks
38.	Distorted steel column (Connecting Bridge).	The building engineer is required to repair the damaged steel column flange with suitable method.	within 6 weeks