

Denim Processing Plant Ltd

Belashahor, Chandina, Comilla-3510
(23.486273, 90.986197)

15th October 2019





Observations



Review of design report



5.1.6 Material:

Steel and Concrete:

Cylinder test data for concrete strength and tensile strength of steel test reports are the followings

For compressive strength of column samples

C1 = 4669 psi
C2 = 4669 psi
C3 = 5234 psi
C4 = 3770 psi
C5 = 4147 psi
C6 = 5275 psi

The equivalent specified concrete strength,

$$f'_{ceq} = 0.9 \bar{f}'_c \left[1 - 1.28 \frac{(k c V)^2}{n - 1} + 0.0015 \right]$$

$\bar{f}'_c$ = Average core strength

Design Report

5.1.6 Material:

Steel and Concrete:

Cylinder test data for concrete strength and tensile strength of steel test reports are the followings

For compressive strength of column samples

C1 = 4860 psi
C2 = 3960 psi

$f'_{ceq} = 3960$ psi (Consideration of minimum Result)

For compressive strength of beam samples

$f'_{ceq} = 3310$ psi

For compressive strength of Slab samples

Design Report-
Converted

Centre for Advisory and Testing Services (CATS-MST)
Military Institute of Science and Technology

Compressive strength of Concrete Cylinders

General Information
CATS Reference: 2379/2018/10/19 Date: 10.10.2018
Client: Dhaka Engineering College & University
Name of the Project: DTPP, Dhaka Engineering College & University
Sample brought by: MD. ABU HASAN
Test Method: ASTM C 39C-10M
Type of Sample: Concrete Cylinders
Location of Sample: Column
Mixture Proportion: 1:1.25
Date of Casting: 14.11.18
Date of Crushing: 19.10.18
Age in days: 230 days
Curing in days: Not Mentioned
Date of Reporting: 19.10.18
Sample Condition: Unstressed
Admixture: No
Design Strength: 3500 psi
Type of Cement: Not Mentioned
Brand of Cement: Premier
Type of Aggregate: Stone chips

Sl. No.	Sample Identification Mark	Length (mm)	Diameter (mm)	Crushing Load (kN)	Crushing Strength (MPa)	Average Strength (MPa)	Failure Type	Fracture Type
1		203	101.0	225.2	22.2		Combined	Shear
2		202	102.0	362.8	35.2	33.5	Combined	Core and Shear
3		202	101.0	280.0	27.7		Combined	Core and Shear

Remarks
1. All information displayed above are provided by the client. CATS-MST could not verify any of those.
2. Samples are received in unsealed condition without signatures from the competent authority. Therefore, authenticity of these samples and their history could not be verified.

Test Supervised By: Jassim Rahman
Counter Signed By: MOHAMMAD NUR HOSSAIN
Colonel, Director, CATS, MST

10/10/2018

Cylinder test report

In design report, the number of concrete samples did not match with the provided report. The calculation of evaluating concrete strength is required to be reviewed following BNBC.



Table for report on Foundation:

SL No	Item	Data		Remark
		Designed	Required	
1	Size and thickness of footing, F1	Size of Footing, F1= (8'-6" x 8'-6") and thickness 24" and 20mm@5"c/c in both direction. Depth of foundation level is 10ft from existing ground level. Allowable bearing capacity of soil is 5.0ksf (F.S. = 2.5).	Size of Footing, F1= (7'-0" x 7'-0") and thickness 20" and 20mm@6"c/c in both direction. Depth of foundation level is 10.0ft from existing ground level. Allowable bearing capacity of soil 5.0ksf (F.S. = 2.5).	Adequate.
2	Size and thickness of footing,	Size of Footing, F2= (12'-0" x 12'-0") and thickness 32" and 20mm@5"c/c in	Size of Footing, F2= (9'-3" x 9'-3") and thickness 23" and 20mm@6"c/c in both direction.	Adequate.

	F2	both direction. Depth of foundation level is 10ft from existing ground level. Allowable bearing capacity of soil is 5.0ksf (F.S. = 2.5).	Depth of foundation level is 10ft from existing ground level. Allowable bearing capacity of soil 5.0ksf (F.S. = 2.5).	
3	Size and thickness of footing, F3	Size of Footing, F3= (9'-9" x 9'-9") and thickness 28" and 20mm@6"c/c in both direction. Depth of foundation level is 10ft from existing ground level. Allowable bearing capacity of soil is 5.0ksf (F.S. = 2.5).	Size of Footing, F3= (9'-3" x 9'-3") and thickness 25" and 20mm@7"c/c in both direction. Depth of foundation level is 10ft from existing ground level. Allowable bearing capacity of soil is 5.0ksf (F.S. = 2.5).	Adequate.
4	Size and thickness of footing, CF1	Size of Footing, CF1= (18'-10" x 16'-5") and thickness 24" and 20mm@6"c/c in both direction. Depth of foundation level is 10ft from existing ground level. Allowable bearing capacity of soil is 5.0ksf (F.S. = 2.5).	Size of Footing, CF1= (12'-3" x 12'-3") and thickness 20" and 20mm@7"c/c in both direction. Depth of foundation level is 10ft from existing ground level. Allowable bearing capacity of soil is 5.0ksf (F.S. = 2.5).	Adequate.
5	Size and thickness of footing, F5	Size of Footing, F5= (10'-0" x 10'-0") and thickness 24" and 20mm@6"c/c in both direction. Depth of foundation level is 10ft from existing ground level. Allowable bearing capacity of soil is 5.0ksf (F.S. = 2.5).	Size of Footing, F5= (9'-6" x 9'-6") and thickness 22" and 20mm@6"c/c in both direction. Depth of foundation level is 10ft from existing ground level. Allowable bearing capacity of soil is 5.0ksf (F.S. = 2.5).	Adequate.

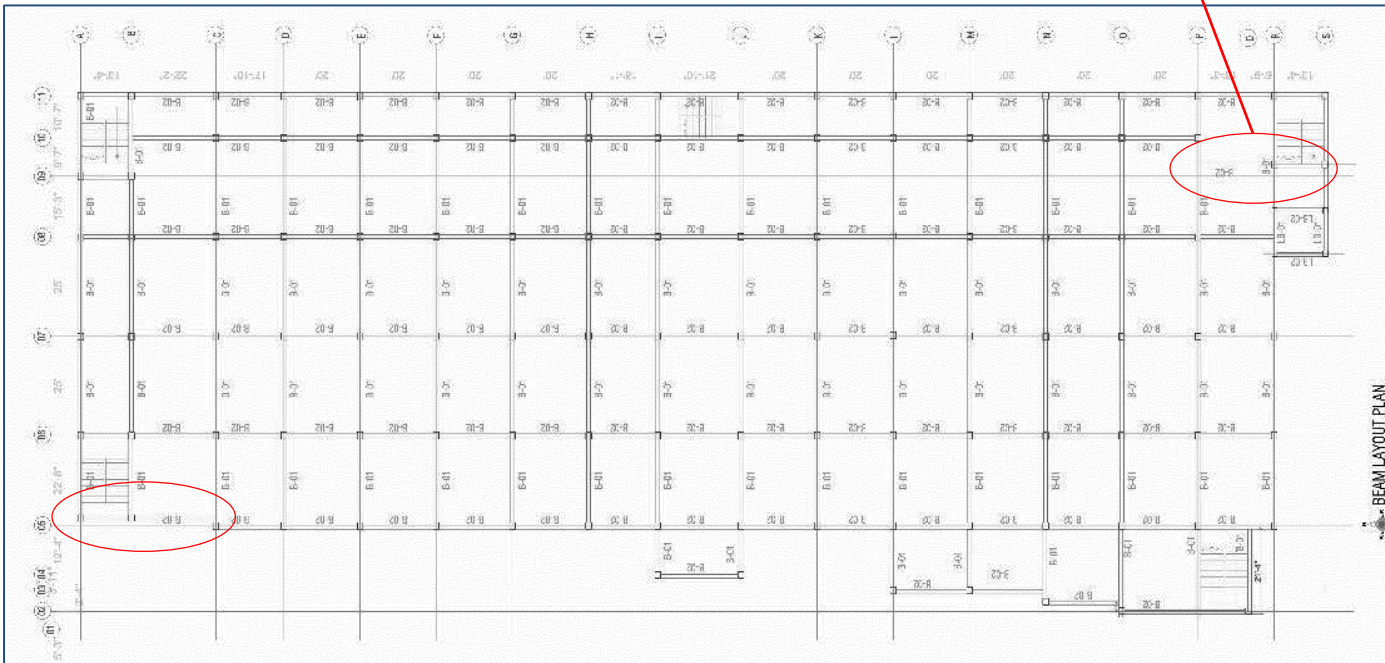
Cursory calculation indicates that the some of the foundations are stressed above design limit considering mentioned bearing capacity of soil. In design report, detail foundation adequacy check, loading information & bearing capacity calculations are not provided.

Observation: RMG building



Discrepancy between as-built drawing and site condition

Observation: RMG building



Beam offset was found in the as-built drawing, but the extension of offset & details was not provided in the drawing.



Water ponding on the roof



Water ponding was found on the roof. Factory is required to improve the drainage system and apply suitable method of water proofing on the roof.



Construction material on roof

Observation: RMG building



Piling up of construction material on the roof may cause overloading. Building engineer required to remove all the construction materials.

Observation: RMG building



Exposed rebar on the roof

Observation: RMG building



Expose rebar prone to corrosion



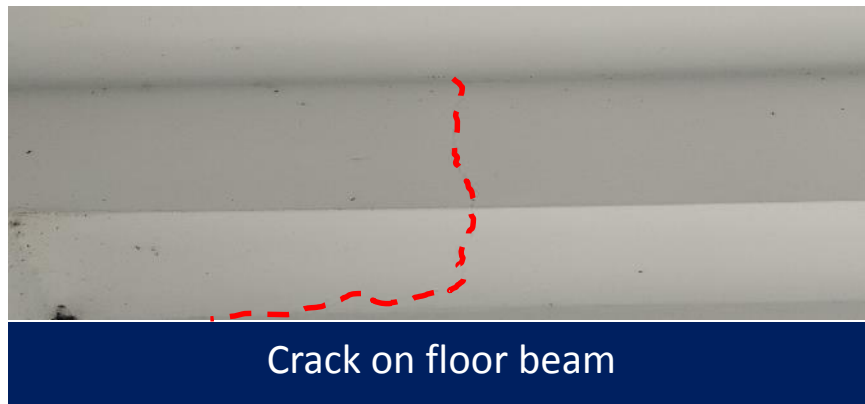
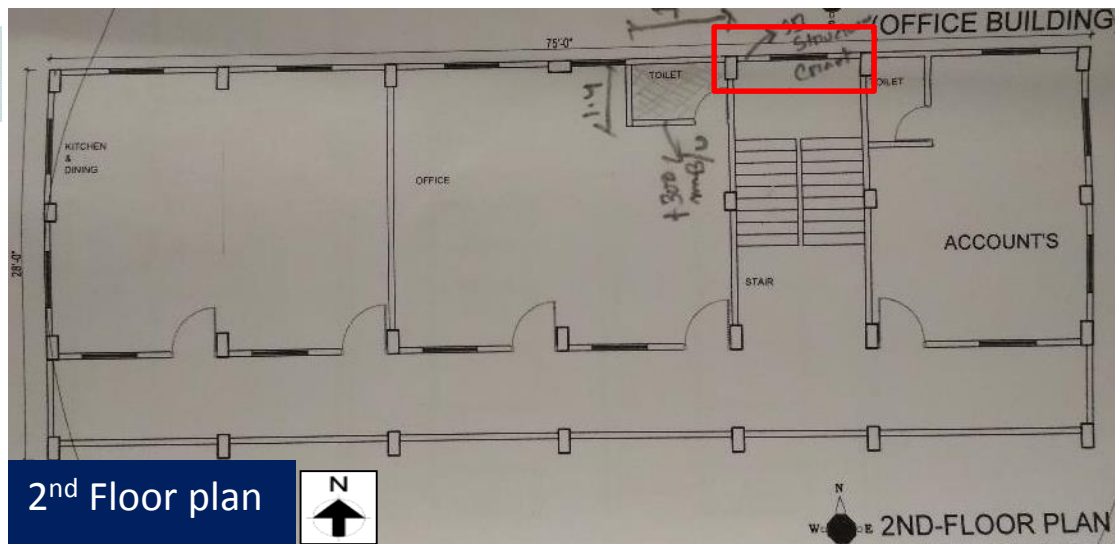
Rebar was found exposed on the roof. Factory is required to take necessary measure to protect the rebar from corrosion.

Observation: RMG building



Crack on beams

Observation: Office building



Crack on floor beam



Structural crack observed on stair beam

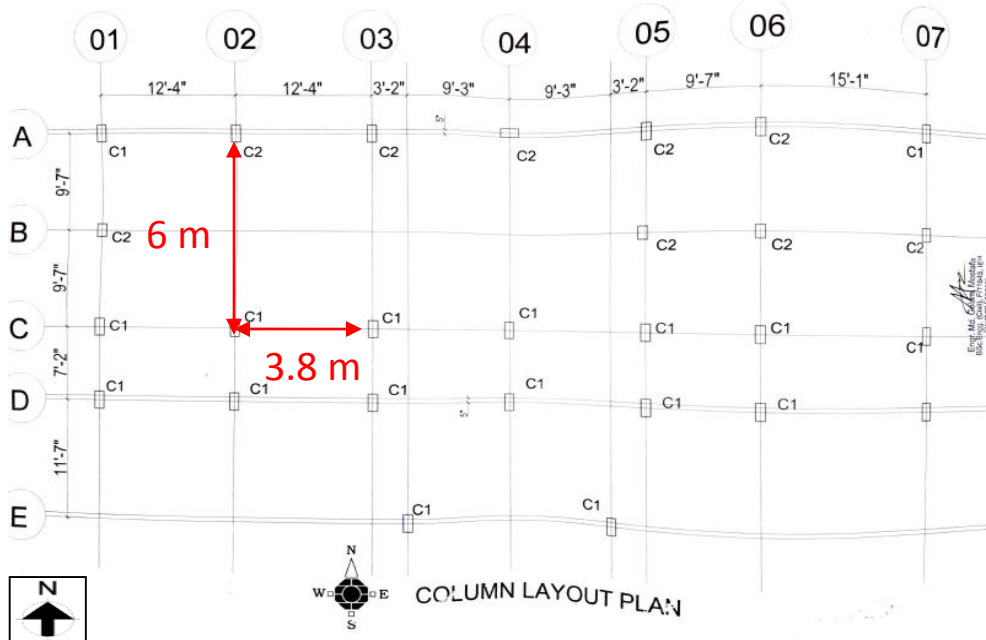
Cracks were observed on floor beams. Building engineer is required to investigate the cracks and carry out suitable repair work as per recommendation of investigation report. Provide temporary prop support to the cracked stair beam until further investigation and completion of remediation.

Observation: Office building



**Cursory calculation indicates column stress above
normal design limit**

Observation : Office building



Column layout plan

Cursory calculation indicates that columns are stressed above normal design limits considering 2 kPa live load on typical floor & minimum concrete strength for brick chips.

COLUMN SCHEDULE

COLUMN NO.	COLUMN SIZE BELOW F.G.L	UPTO G.F TO ROOF	TIE BAR
C1	12" x 12" 	10" x 10" 4-T16	 10mm @ 4"-6"
C2	12" x 17" 	10" x 15" 6-T16	 8mm @ 4"-6"

C1 column-4-16 mm bar & rebar strength considered 275 MPa.



Brick chips aggregate

Observation : Office building



Lack of lateral stability system

Observation : Shed 2



No lateral stability system (roof bracing, wall bracing, compression strut) observed. Building engineer is required to carry out an engineering assessment to check the lateral stability of the shed.



Non engineering shed

Observation : Shed 3



Apparently non engineering shed: poor connection details, inadequate member sizes and corroded steel members.



Problems Observed

RMG building :

- 1: Review of design report.
- 2: Discrepancy between as-built drawing and site condition.
- 3: Construction debris on the roof.
- 4: Water ponding on the roof.
- 5: Exposed rebar susceptible to corrosion.

Office building :

- 6: Crack on beams.
- 7: Cursory calculation indicates column stress above normal design limit.

Shed-2

- 8: Lack of lateral stability system.

Shed-3 :

- 9: Non engineering shed.



Priority Actions



Item No.	Observation	Recommended Action Plan	Recommended Timeline
1	Review of design report. (RMG building)	Building engineer is required to review the design report including the foundation design, load and stresses.	6-weeks
2	Review of design report. (RMG building)	Carry out any remedial works where necessary.	6-months
3	Discrepancy between as-built drawing and site condition. (RMG building)	Building engineer is required to survey the structure and update the as-built drawings.	6-weeks
4	Construction debris on the roof. (RMG building)	Remove all the construction debris from the roof.	Immediate - Now
5	Construction debris on the roof. (RMG building)	Implement better supervision of loadings in all level.	6-weeks
6	Water ponding on the roof. (RMG building)	The roof slab drainage system should be improved. In addition waterproofing material is to be applied on the roof slab.	6-weeks
7	Exposed rebar susceptible to corrosion. (RMG building)	Factory is required to take necessary measure to protect the rebar from corrosion.	6-weeks



Item No.	Observation	Recommended Action Plan	Recommended Timeline
8	Crack on beams. (Office building)	Provide temporary prop support to the cracked beam.	Immediate - Now
9	Crack on beams. (Office building)	Building engineer is required to carry out the investigation and suggest remedial works.	Immediate - Now
10	Crack on beams. (Office building)	Carry out suitable repair works as per investigation report.	6-weeks
11	Cursory calculation indicates column stress above normal design limit. (Office building)	Factory Engineer to review design, loads and columns stresses.	6-weeks
12	Cursory calculation indicates column stress above normal design limit. (Office building)	Verify in-situ concrete stresses by 100mm dia. cores from min. 4 no. column from lower tier.	6-weeks
13	Cursory calculation indicates column stress above normal design limit. (Office building)	Produce and actively manage a loading plan for all floor plates within the factory considering floor capacity and column capacity.	6-weeks
14	Cursory calculation indicates column stress above normal design limit. (Office building)	Carry out remedial works where necessary.	6-months



Item No.	Observation	Recommended Action Plan	Recommended Timeline
15	Lack of lateral stability system. (Shed-2)	Building engineer is required to check the lateral stability & serviceability against the lateral load as per BNBC.	6-weeks
16	Lack of lateral stability system. (Shed-2)	Carry out remedial works where necessary.	6-months
17	Non engineering shed. (Shed-3)	Building engineer is required to suggest any alteration/demolition of the shed.	6-weeks
18	Non engineering shed. (Shed-3)	Carry out alteration/demolition works.	6-months